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EFFECT OF SOIL - STRUCTURE INTERACTION ON SEISMIC RESPONSE OF BUILDING

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ABSTRACT

The investigation on the energy transfer mechanism from soil to buildings during earthquake is critical for the design of earthquake resistant structure, and also current demand of high rise and economical building is raising, thus the necessity of research into soil structure interaction problem is greater than ever. Generally in the analysis of structures which is subjected to seismic forces also, is usually assumed that the structure is fixed at the base to simplify the mathematical problem. This assumption leads to an error in assessment of overall response under seismic loads. The interaction phenomenon is principally affected by the mechanism of energy transfer between the structure and soil during the earthquake. The influence of soilstructure interaction on seismic response of such high rise building is a major concern to incorporate the necessary change in the designing such structures. Hence in this study, threedimensional FEM model is constructed to analyze the effect of different soil conditions and number of stories on the vibration characteristics and seismic response demands of building structures. Numerically results obtained using soil structure interaction model conditions are compared to those corresponding to fixed-base support conditions. The peak responses of story shear, story moment, story displacement, story drift, moment at beam ends, as well as force of inner column are analyzed. The analysis result of different approaches is used to evaluate the advantages, limitations, and ease of application of each approach for seismic analysis.

KEYWORDS

Soil Structure Interaction; RCC; Soil Spring constant; Earthquake analysis; Shear wave velocity.

1. INTRODUCTION

The rapid development of urban population and the pressure on limited space significantly influence the residential development of the city. The price of the land is high, the desire to avoid uneven and uncontrolled developing of urban area and bear on the land for needs of important agricultural production activity have all led to route residential building upwards. The local topographical restrictions in the urban area only possible solutions for construction of multi-story buildings to full fill the residential needs. The multi-storey buildings all initially a reaction to the demand by activity of business close to each other and in city center, the less availability of land in the area. The multi-storey buildings are frequently developed in the center of the city is prestige symbols for commercial organizations. Further the tourist and business community. The soil structure interaction is a special field of analysis in earthquake engineering, this soil structure is influenced by the motion of structure interaction".

2. METHODOLOGY

The present work attempts to study the behavior of framed structures with rigid and flexible foundation. Framed structure of different height with symmetrical and irregular plans have been considered with fixed and flexible foundation resting on three different types of soil and different types of foundation. A framed structure of rectangular regular and irregular plan with10 story is analyzed for earthquake load consider in zone-IV, importance factor of 1.5, with the different soil type like hard, medium and soft soil with fixed and flexible base condition. Static analysis for 10 storied structure is done and the parameters like time period, base shear, bending moment in column and top story displacement are measured and are present below. In the flexible base condition the soil and foundation is modeled as soil spring. The stiffness of spring is calculated based on soil properties and foundation details using empirical formulae.

All the building models are analyses in ETABS. The properties of the building configurations are considered in the present work are summarized below.

2.1 SPECIFICATION OF THE BUILDING

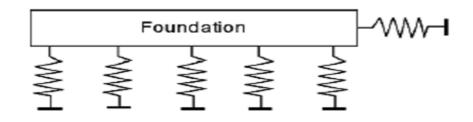
- Height of each floor: 3m and Plan dimension: 18x18m
- Floor thickness: 0.125m and Wall thickness: 230mm
- Compressive strength of concrete fck = 30 N/mm^2 and steel used Fe = 500 N/mm^2
- Density of concrete: 25 kN/m² and Brick: 20 kN/m²
- Size of column: 3000mmx300mm to 650mmx650mm as per the structural requirement
- Size of beam: 230mmx600mm
- Seismic zone factor 'z' = (IV) and Damping ratio= 0.05
- Response reduction factor 'R'=3 and Importance factor 'I'=1.5
- Live load on top story: 4kN/m2and on remaining story: 4 kN/m²
- The floor finish load is: 1.5 kN/m^2
- Wall load at floor is: 11.00kN/m and for parapet is: 4.6kN/m

2.2 Details of soil parameters considered.

| Sr. no. | Soil type | | S | oil Param | eters | |
|------------|----------------------------------|------------------------------|--|--------------------------------|----------------------------------|---|
| | | Poisons ratio' v ' | Modulus Of elasticity'E' Kn/Sq.m | Mass density 'p' Kn/Cu.m | Soil S.B.C. Kn/Sq.m | Modulus of subgrade reaction 'Ks' Kn/Cu.m |
| 1 | Hard soil (highly dense sand) | 0.3 | 60000 | 20 | 400 | 64000 |
| 2 | Medium soil (sandy clay) | 0.3 | 25000 | 19 | 200 | 32000 |
| 3 | Soft soil (Silty sand) | 0.35 | 15000 | 18 | 100 | 24000 |

Table no. - 01

2.3 Equivalent Spring Constant





As per Winkler's model the soil medium is represented by number of identical but mutually independent, closely spaced, discrete linearly elastic springs. The movement of foundation is generally considered in two perpendicular horizontal directions and in vertical direction. The rotation of the same about these three directions should also be considered as shown in fig..... For building with isolated footing, below each column, three translation springs along three directions and three rotational spring about those mutually perpendicular axis should be put together to simulate the effect of soil flexibility,

| Degree of Freedom | Stiffness of Foundation at Surface | Note |
|--------------------------|--|---------------------------------|
| Translation along x-axis | $K_{x, sur} = \frac{GB}{2 - v} \left[3.4 \left(\frac{L}{B}\right)^{0.65} + 1.2 \right]$ | • |
| Translation along y-axis | $K_{y, sur} = \frac{GB}{2-v} \left[3.4 \left(\frac{L}{B}\right)^{0.65} + 0.4 \frac{L}{B} + 0.8 \right]$ | bottom |
| Translation along z-axis | $K_{z,sur} = \frac{GB}{1-v} \left[1.55 \left(\frac{L}{B}\right)^{0.75} + 0.8 \right]$ | center x |
| Rocking about x-axis | $K_{xx, sur} = \frac{GB^3}{1-v} \left[0.4 \left(\frac{L}{B} \right) + 0.1 \right]$ | l 2 B |
| Rocking about y-axis | $K_{yy, sur} = \frac{GB^3}{1-v} \left[0.47 \left(\frac{L}{B}\right)^{2.4} + 0.034 \right]$ | Orient axes such that $L \ge B$ |
| Torsion about z-axis | $K_{zz, sur} = GB^{3} \left[0.53 \left(\frac{L}{B} \right)^{2.45} + 0.51 \right]$ | |

Table no. - 02

Also correction factor for depth of footing is applied to the each spring constant values, calculated from above formulas.

2.3.1 CASE A- Regular 10 story model

1) Condition 1 - Hard soil (highly dense sand, SBC = 40 T)

| Column | | Colu | | Dept | | | | SPRING | VALLU | E | |
|--------------------------|------------------|--------------------------|-------------------|-------------------------------------|---|--------|--------|--------|--------|----------|---------|
| 0. | Colum n Sizes | mn force s 'KN' | Footin g sizes | h of Footi ng ' D ' | Thickn ess of footing ' d ' | Kx | Ку | Kz | Kxx | Куу | Kzz |
| | | | | | | | | | | | |
| C1,4,5,8 | 450X450 | 1723 | 2.1x2.1 | 2 M | 400 mm | 319575 | 31975 | 229593 | 263554 | 306504 | 482118 |
| C2,3,6,7,13 ,14,15,16 | 600X600 | 3103 | 2.8x2.8 | 2 M | 400 mm | 373112 | 373112 | 285542 | 542021 | 641331 | 1002293 |
| C9,10,11,1 2 | 700X700 | 5004 | 3.5x3.5 | 2 M | 400 mm | 425335 | 425335 | 341413 | 972334 | 1162307 | 1788646 |
| | | | | | | | | | | | |

Table no. - 03Soil Spring constant

| Column | | Colu | | Dept | | | | SPRING | VALLU | E | |
|--------------------------|------------------|--------------------------|-------------------|-------------------------------------|---|--------|--------|--------|---------|---------|---------|
| no. | Colum n Sizes | mn force s 'KN' | Footin g sizes | h of Footi ng ' D ' | Thickn ess of footing ' d ' | Kx | Ky | Kz | Kxx | Куу | Kzz |
| | 11 51205 | IXIV | 5 51205 | D | u | 111 | ту | 112 | IXAA | туу | IXLL |
| C1,4,5,8 | 450X450 | 1723 | 3x3 | 2 M | 400 mm | 161728 | 161728 | 125631 | 269996 | 320610 | 498988 |
| C2,3,6,7,13 ,14,15,16 | 600X600 | 3103 | 4x4 | 2 M | 400 mm | 192519 | 192519 | 158857 | 579774 | 695861 | 1058970 |
| C9,10,11,1 2 | 700X700 | 5004 | 5x5 | 2 M | 400 mm | 222651 | 222651 | 191998 | 1068124 | 1285982 | 1919413 |
| | | | | | | | | | | | |

2) Condition 2 -Medium soil (Sandy clay sand SBC = 20 T)

Table no.- 04Soil Spring constant

3) Condition 3 - Soft soil (Silty sand SBC = 10 T)

| Column | | Colu | | Dept | | | | SPRING | VALLU | E | |
|--------------------------|------------------|-------------|-------------------|--------------------|-------------------------|--------|--------|--------|---------|---------|---------|
| no. | | mn force | | h of Footi | Thickn ess of | | | | | | |
| | Colum n Sizes | s 'KN' | Footin g sizes | ng ' D ' | footing ' d ' | Kx | Ky | Kz | Kxx | Куу | Kzz |
| L | 1 | | 0 | | 1 | | | | | | |
| C1,4,5,8 | 450X450 | 1723 | 4.15x4.1 5 | 2 M | 400 mm | 117316 | 117316 | 101941 | 398618 | 478827 | 674486 |
| C2,3,6,7,13 ,14,15,16 | 600X600 | 3103 | 5.6x5.6 | 2 M | 400 mm | 143165 | 143165 | 131815 | 910928 | 1096635 | 1505367 |
| C9,10,11,1 2 | 700X700 | 5004 | 7.0x7.0 | 2 M | 400 mm | 167651 | 167651 | 160575 | 1708233 | 2050332 | 2765792 |
| | | | | | | | | | | | |

Table no.- 05Soil Spring constant

2.3.2 CASE B- Irregular 10 story model

1) Condition 1 - Hard soil (highly dense sand SBC = 40 T)

| Column | | Colu | | Dept | | | | SPRING | VALLU | E | |
|----------------------------|------------------|-------------|-------------------|--------------------|-------------------------|--------|--------|--------|--------|---------|---------|
| no. | | mn force | | h of Footi | Thickn ess of | | | | | | |
| | Colum n Sizes | s 'KN' | Footin g sizes | ng ' D ' | footing ' d ' | Kx | Ky | Kz | Kxx | Vww | Kzz |
| | II DIZCS | IXIV | g 51205 | D | u | ТА | Ку | ΝL. | МЛЛ | Куу | INLL |
| C8,17,18,2 | 1 | | | 1 | | | | | | | |
| 0,22 | 450X450 | 2726 | 2.1x2.1 | 2 M | 400 mm | 319575 | 31975 | 229593 | 263554 | 306504 | 482118 |
| C1,3,5,6,7, 13,14,15,19 | | | | | | | | | | | |
| ,21 | 600X600 | 4792 | 2.8x2.8 | 2 M | 400 mm | 373112 | 373112 | 285542 | 542021 | 641331 | 1002293 |
| C2,9,10,11, 12,16 | 700X700 | 7620 | 3.5x3.5 | 2 M | 400 mm | 425335 | 425335 | 341413 | 972334 | 1162307 | 1788646 |
| 12,10 | , | . 020 | 0.0.1010 | | | .20000 | .20000 | 5.1115 | 2.2001 | 1132307 | 1,00010 |
| | | | | | | | | | | | |

Table no.- 06Soil Spring constant

| Column | | Colu | | Dept | | | | SPRING | VALLU | E | |
|----------------------------|---------|-------------|---------|---------------|------------------|--------|--------|--------|---------|---------|---------|
| no. | | mn force | | h of Footi | Thickn ess of | | | | | | |
| | Colum | S | Footin | ng | footing | • | •7 | •7 | 17 | • | 17 |
| | n Sizes | 'KN' | g sizes | ' D ' | 'd' | Kx | Ky | Kz | Kxx | Куу | Kzz |
| | | | | | | | | | | | |
| C8,17,18,2 0,22 | 450X450 | 2726 | 3x3 | 2 M | 400 mm | 161728 | 161728 | 125631 | 269996 | 320610 | 498988 |
| C1,3,5,6,7, 13,14,15,19 | | | | | | | | | | | |
| ,21 | 600X600 | 4792 | 4x4 | 2 M | 400 mm | 192519 | 192519 | 158857 | 579774 | 695861 | 1058970 |
| C2,9,10,11, 12,16 | 700X700 | 7620 | 5x5 | 2 M | 400 mm | 222651 | 222651 | 191998 | 1068124 | 1285982 | 1919413 |
| | | | | | | | | | | | |

2) Condition 2 -Medium soil (Sandy clay sand SBC = 20 T)

Table no.- 07Soil Spring constant

3) Condition 3 - Soft soil (Silty sand SBC = 10 T)

| Column | | Colu | | Dept | Thick | | | SPRING | VALLU | E | |
|----------------------------|---------|-------|-----------|--------------|----------------|--------|--------|--------|---------|---------|---------|
| no. | | mn | | h of | ness | | | | | | |
| | | force | | Footi | of | | | | | | |
| | Colum | S | Footin | ng | footin | | | | | | |
| | n Sizes | 'KN' | g sizes | ' D ' | g ' d ' | Kx | Ky | Kz | Kxx | Куу | Kzz |
| | | | | | | | | | | | |
| C8,17,18,2 0,22 | 450X450 | 2726 | 4.15x4.15 | 2 M | 400 mm | 117316 | 117316 | 101941 | 398618 | 478827 | 674486 |
| C1,3,5,6,7, 13,14,15,19 | | | | | | | | | | | |
| ,21 | 600X600 | 4792 | 5.6x5.6 | 2 M | 400 mm | 143165 | 143165 | 131815 | 910928 | 1096635 | 1505367 |
| C2,9,10,11, 12,16 | 700X700 | 7620 | 7.0x7.0 | 2 M | 400 mm | 167651 | 167651 | 160575 | 1708233 | 2050332 | 2765792 |
| | | | | | | | | | | | |

Table no.- 08Soil Spring constant

According to above structural and loading specification, structural models are prepared as shown below,

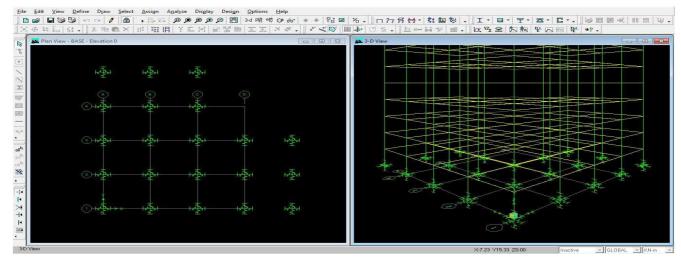


Fig. no 02Etabs Analysis model

3. RESULT AND DISCUSSION.

A sample representation of base shear, displacement for 10 story fixed footing model as per software represents is shown below in fig

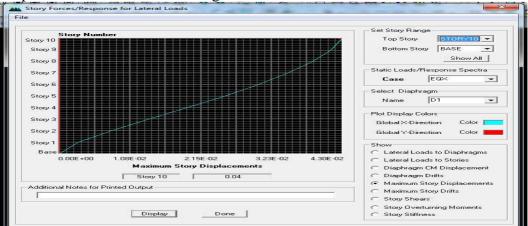


Fig. no. 03- Story Discplacement Diagram (10 st. regular shaped with fixed footing)

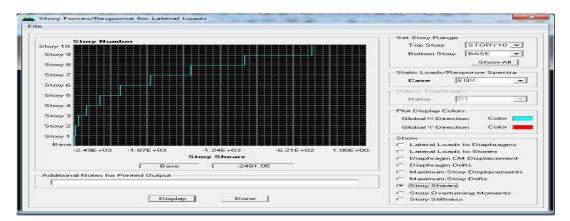


Fig. no. 04- Story Shear Diagram (10 st. regular shaped with fixed footing)

Similarly, story shear and story Displacement results for regular shaped but flexible footing also shown below,

3.1. 10 story Regular shaped structure standing on different type of footings.

| | | | STRU | CTURAL PARA | METERS |
|-------------------|-------------------|-------------------------|--------------------|---------------------|-------------------|
| STRUCTURE CASE | SOIL CONDITION | FOUNDATION CONDITION | Base shear (KN) | Time Period(Sec) | Deflection(mm) |
| CASE | CONDITION | CONDITION | (IXIV) | Tellou(See) | Deficetion(initi) |
| | HARD | FIXED | 1683.71 | 1.4718 | 41.64 |
| | | FLEXIBLE | 1563.46 | 1.5851 | 44.5 |
| 10 STORY | | | | | |
| | MEDIUM | FIXED | 2283.64 | 1.4718 | 56.42 |
| REGULAR | | FLEXIBLE | 2052.976 | 1.6461 | 63.85 |
| SHAPE | | | | | |
| | SOFT | FIXED | 2811.792 | 1.4718 | 69.05 |
| | | FLEXIBLE | 2487.793 | 1.6666 | 79.8 |

Table no.- 09Results for 10 stories regular shape structure.

Graphical representation of above results.

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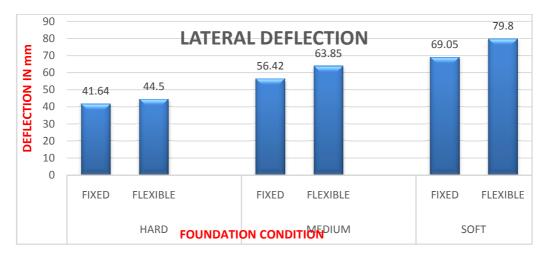


Fig. no. 05- Lateral Deflection for 10 stories regular shape structure.



Fig. no. 06- Base shears Comparison for 10 stories regular shape structure.

3.2. Summary of results for 10 stories Irregular shaped structure standing on different type of footings.

| | | | STRU | CTURAL PARA | METERS |
|-----------|---------------|------------|-------------|-------------|----------------|
| STRUCTURE | SOIL | FOUNDATION | Base shear | Time | |
| CASE | CONDITION | CONDITION | (KN) | Period(Sec) | Deflection(mm) |
| | | | | | |
| | HARD | FIXED | 2228.645 | 1.4845 | 42.38 |
| | | FLEXIBLE | 2092.73 | 1.5828 | 46.7 |
| 10 STORY | | | | | |
| | MEDIUM | FIXED | 3022.74 | 1.4845 | 57.89 |
| IRREGULAR | | FLEXIBLE | 2763.561 | 1.6447 | 66.85 |
| SHAPE | | | | | |
| | SOFT | FIXED | 3721.84 | 1.4845 | 70.85 |
| | T 11 1 | FLEXIBLE | 3362.67 | 1.666 | 83.9 |

Table no.-10 Results for 10 stories Irregular shape structure.

Graphical representation of above results.

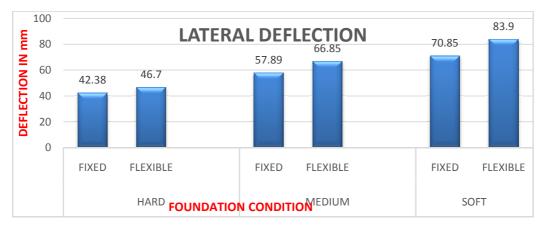


Fig. no. 07- Lateral Deflection for 10 stories Irregular shape structure.



Fig. no. 08- Base shears Comparison for 10 stories Irregular shape structure.

4. DISCUSSION

As per the above observations, comparative calculation for various factor has been worked out for further study of structural behaviour due to soil structure interaction.

Some of the comparative factor has worked out and tabulated for simplifications of assessment

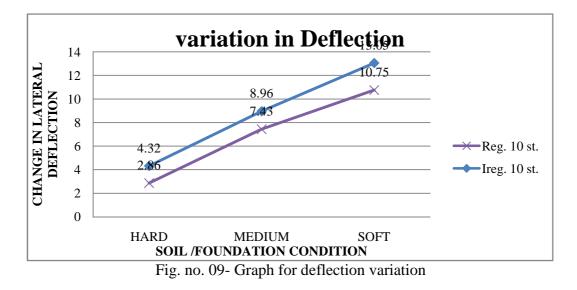
| SR. | STRUCTURE TYPE | % CHAN | IGE IN BASE | E SHEAR | CHANGE IN DEFLECTION | | | |
|-----|-----------------|--------|-------------|---------|----------------------|--------|-------|--|
| NO. | | HARD | MEDIUM | SOFT | HARD | MEDIUM | SOFT | |
| | | | | | | | | |
| 1 | 10 STORY REG. | 7.15 | 10.1 | 11.53 | 2.86 | 7.43 | 10.75 | |
| 2 | 10 STORY IRREG. | 6.1 | 8.57 | 9.65 | 4.32 | 8.96 | 13.05 | |

Table no.-11 Overall Result comparison

As shown in the above table, Percentage changes of seismic base shear and Displacement for the entire case are summaries; these variations are among fixed rigid footing and Flexible footings representing respective soil condition.

Graphical representation of variation in story deflection of the entire Structural model configuration

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Graphical representation of variation in Base shear of all the Structural model configuration.

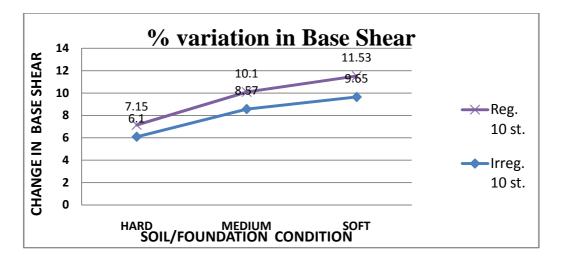


Fig. no. 10- Graph for Base shear variation

5. CONCLUSION

The study as a whole may prove useful in formulating design guidelines for seismic design of building frames incorporating the effect of soil-flexibility.

1) Fundamental natural period and Base shear

- The fundamental natural period of a specific structures and their base shear are increases as soil foundation moves on hard soil to Soft soil.
- The fundamental natural period of specific structures and their base shear are increases as soil foundation moves on Rigid footing to Flexible footing.
- Natural frequency of the structure reduces as foundation condition changes from fixed to flexible.

- Variation in time period in irregular shaped building is quite more than compare to regular shaped building.
- The variation of base shear of the structures between Rigid and flexible footing are increases as soil condition changes hard to soft.
- Variation in Base shear in irregular shaped building is quite lesser than compare to regular shaped building.

2) Maximum lateral displacement

- Structures shows more deflection as soil foundation moves on hard soil to Soft soil.
- The variation in the deflection of the structures with Rigid and with flexible footing is comparatively higher as soil condition changes hard to soft.
- Lateral displacement of the structures increases as foundation condition changes from Fixed Rigid footing to Flexible footing.
- Variation in deflection in irregular shaped building is higher than compare to regular shaped building.

3) Change in Superstructure reaction

- Due to the application of column moments at the base is reduced.
- Reaction of beams also changes by some minor extent.
- The variation of reaction in elements is more in case of soft soil condition.

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